

59th Annual Meeting



Maximizing Pavement Life Considerations for Pavement Design and Construction

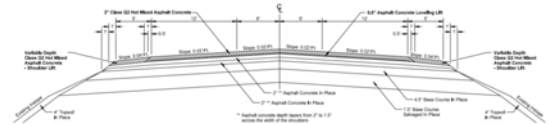
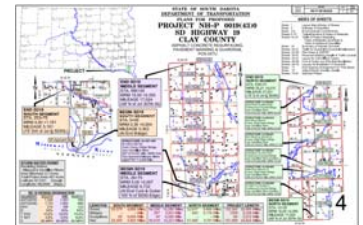
Madison, Wisconsin
Nov. 27th, 2018

Mark D. Blow P.E.
Sr. Regional Engineer
Asphalt Institute

Pavement Design Reminders



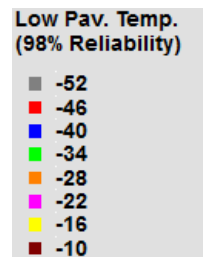
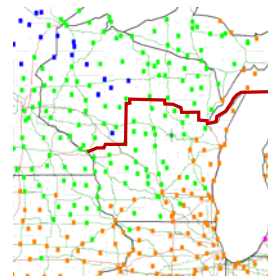
- Binder Type
- Pavement Thickness
- Lift Thickness & Mix Type



Prevent Thermal Cracking



- Use PG -34 Binders on New Construction
- 25 year old SHRP recommendation



LTPPBIND v3.1

Binder Type

WisDOT Binder regions



LOWER LAYERS:
58-28 S

OVERLAYS:
58-28 S, H, or V**

UPPER LAYERS:
Southern Asphalt Zone:
58-28 S, H, or V**

Northern Asphalt Zone:
58-34 S, H, or V**



- * New construction, reconstruction, and pavement replacement projects (Northern Zone-Upper Layer Only)
- ** V binder designation is to be used on >8 Million ESAL projects with slow moving traffic and SMA projects
- *** E binder designation is to be used on >30 Million ESAL projects

Iowa HIPRO Thin Lifts



Utilizing 58-28 E(+)
% Recovery >90%

On overlays



Consider high performance binders on overlays

- Reduced rutting
- Reduced cracking
- Better crack seal performance
- Maintain existing crack resistance

PG 64-22 modified, no rutting

PG 67-22 unmodified, 15mm rut

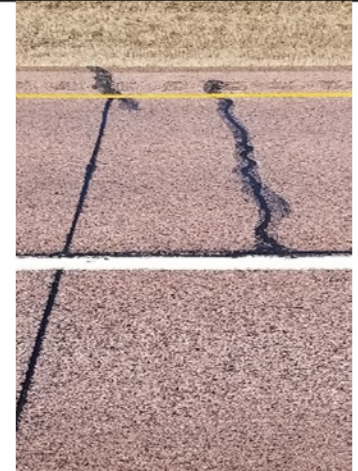


Better Joint Performance



I-29 Project

- Orig. 1962 PCC
- 1999 - 3" HMA
- 2004 – 1.5" SMA
✓ PG 70-28
- 2004 – Saw/Seal
- 2009 – Sealed secondary cracks



But what does it cost?



2016 DOT Ave. Unit Bid Prices

Description	Unit	Total Quantity	Average Bid Price
PG 58-28 Asphalt Binder	Ton	15,547.30	\$433.74
PG 58-34 Asphalt Binder	Ton	125,661.00	\$485.86
PG 64-22 Asphalt Binder	Ton	519.60	\$470.63
PG 64-28 Asphalt Binder	Ton	182,175.80	\$457.38
PG 64-34 Asphalt Binder	Ton	61,502.20	\$499.15

Highest to Lowest Difference = \$65.41/ binder ton
@ 6% Binder = \$3.92 / ton of mix

Simple county overlay



10 mile overlay, No milling

Item	Quantity	Units	Unit Price	Amount	% of Project Cost
PG64-28	1,575.50	Ton	\$ 462.07	\$ 727,991.29	48.5%
Class E Mix	25,627.20	Ton	\$ 22.82	\$ 584,812.70	39.0%
				Tot. Mix Cost = \$ 1,312,803.99	87.5%
				Tot Project Cost = \$ 1,500,814.57	100.0%

Assuming a \$65.41 binder cost increase,

		Expected Service Needed increase in Life (Years)	Life (Years)
Binder cost increase	14.2%	15	1.0
Project cost increase	6.9%	20	1.4
		25	1.7

Complicated Interstate overlay



20 + mile overlay, underdrains etc.

Item	Quantity	Units	Unit Price	Amount	% of Project Cost
PG64-28	1,823.80	Ton	\$ 400.00	\$ 729,520.00	3%
PG64-34	11,152.60	Ton	\$ 430.00	\$ 4,795,618.00	21%
				Tot Project Cost = \$ 22,565,024.00	

Assuming a \$65.41 binder cost increase,

		Expected Service Needed increase in Life (Years)	Life (Years)
Binder cost increase	15.4%	15	0.6
Project cost increase	3.8%	20	0.8
		25	0.9

Smaller Urban / Grading



1 mile 3-Lane, grading, C&G, sidewalks, lighting and new asphalt surface

Item	Quantity	Units	Unit Price	Amount	% of Project Cost
PG58-28	447.70	Ton	\$ 730.00	\$ 326,821.00	9.3%
Class HR	9,518.50	Ton	\$ 33.00	\$ 314,110.50	8.9%
				Tot. Mix Cost = \$ 640,931.50	18.2%
				Tot Project Cost = \$ 3,521,707.00	

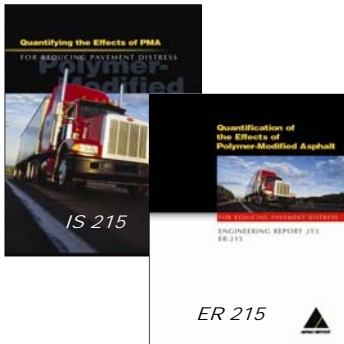
Assuming a \$65.41 binder mix cost increase,

		Expected Service Needed increase in Life (Years)	Life (Years)
Binder cost increase	9.0%	15	0.12
Project cost increase	0.8%	20	0.17
		25	0.21

LTTP Studies



Quantifying the Effects of PMA for Reducing Pavement Distress



This study (published in Feb 2005) used national field data to determine enhanced service life of pavements containing polymer modified binders versus conventional binders. The data is from a variety of climates and traffic volumes within North America.

13

Summary of Expected Increase in Service Life, Years, Based on M-E Damage Based Analysis



Assumptions: Unmodified sections designed for 20 yr. life. Also, PMA in top 4 inches.

Site Factor	Condition Description		Added Life
Foundation	Non-expansive, coarse soils		5-10
	Expansive and plastic soils (PI>35)		2-5
	Frost Susceptible in cold climate		2-5
Water Table & Drainage	Deep		5-10
	Shallow; adequate		5-8
	Shallow; inadequate		0-2
Existing Pavement Condition	HMA	Good	5-10
		Poor-extensive cracking	1-3
	PCC	Good	3-6
		Poor-faulting & cracking	0-2

Continued: Summary of Expected Increase in Service Life, Years

Assumptions: Unmodified sections designed for 20 yr. life. Also, PMA in top 4 inches.

Site Factor	Condition Description		Added Life
Climate; Temp. Fluctuations	Hot	Hot Extremes	5-10
	Mild		2-5
	Cold	Cold Extremes	3-6
Traffic, Truck Volumes	Low	Intersections	5-10
		Thoroughfares	3-6
		Heavy Loads	5-10
	Moderate		5-10
	High		5-10

Recycled Binders



NCHRP Project D9-12



NCHRP REPORT 452 - Results

- Blending occurs at higher RAP contents. At low RAP contents, effects are not significant.
- Results from all phases support concept of a tiered system.
 - Mix ETG recommendations were largely confirmed.

NCHRP 9-12 Recommendations



RAP mixtures should be able to perform at least as well as virgin mixes.

ACTION	RAP
No Change in Binder Grade	15% or less
One Grade Lower	16 - 25%
Use Blending Charts	>25%

Adopted in AASHTO M323
Superpave Volumetric Mix Design

Acknowledgement



WHRP
Wisconsin Highway Research Program



- Project 0092-14-06 Critical Factors Affecting Asphalt Durability
 - Evaluate changes to the composition of asphalt mixtures that WisDOT should consider to improve durability
 - Resistance to load associated cracking
 - Resistance to aging

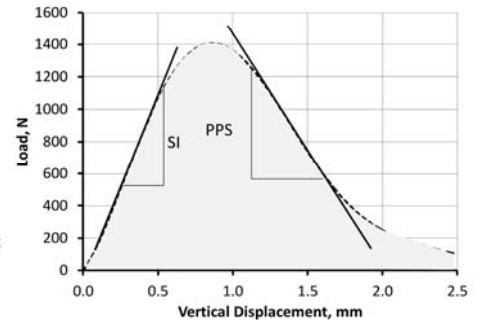
<http://wisconsin.gov/documents2/research/14-06-revised-final-report.pdf>



Illinois SCB (Flexibility Index)



SI = Stiffness Index
Slope @ 50% of Peak Load
AI=Aging Index
 $AI = SI_{LTOA} / SI_{STOA}$



FI = Energy/Post Peak Slope
Resistance to cracking increases with increasing FI



Effect of Recycled Binder

No Grade Change

AASHTO M 323 Change

VBE	Low Grade	RBR	FI	% of Control Life	VBE	Low Grade	ABR	FI	% of Control Life
10.5	-28	0.00	6.5	100	10.5	-28	0.00	6.5	100
10.5	-28	0.05	6.0	92	10.5	-28	0.05	6.0	92
10.5	-28	0.10	5.5	84	10.5	-28	0.10	5.5	84
10.5	-28	0.15	5.0	77	10.5	-28	0.15	5.0	77
10.5	-28	0.20	4.5	69	10.5	-34	0.20	6.9	105
10.5	-28	0.25	4.0	61	10.5	-34	0.25	6.3	97
10.5	-28	0.30	3.5	53	10.5	-34	0.30	5.8	89



TURNER-FAIRBANK HIGHWAY RESEARCH CENTER

Project #1: High RAP (RAS) + WMA Accelerated Pavement Test



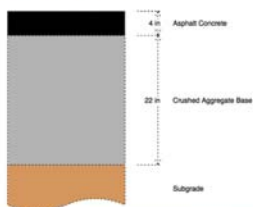
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TURNER-FAIRBANK HIGHWAY RESEARCH CENTER

The Experiment

Structure

- 10 Lanes (10 Mixes)
- Build in 2013



Materials

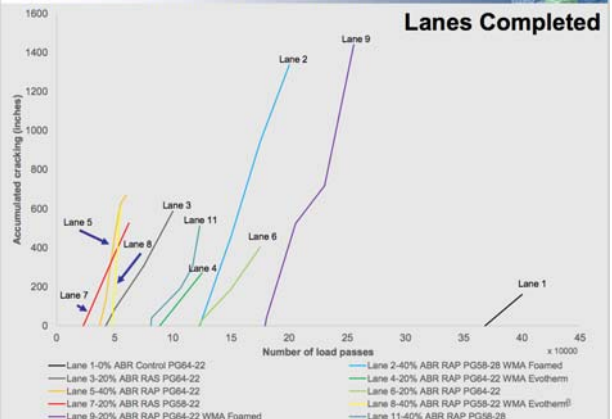
- 2 Binder Grades
- RAP/RAS
- 2 WMA Technologies
- 3 ABR contents



4

TURNER-FAIRBANK HIGHWAY RESEARCH CENTER

Lanes Completed



Moderation is the key



"I LIMIT MYSELF TO ONE GLASS OF WINE A DAY."

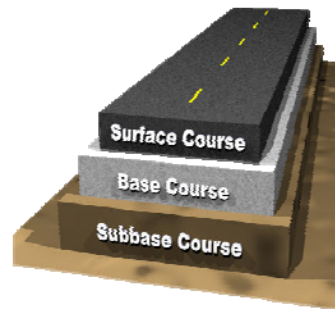
Thickness Design

Design - Section 2

Pavement Design



MEPDG (AASHTO 2000)



AASHTO has been developing MEPDG for high volume roads, but a gap has developed for local roads and lower volume roads.

www.PaveXpressDesign.com 28

What Is PaveXpress?



A free, online tool to help you create simplified pavement designs using key engineering inputs, based on the AASHTO 1993 and 1998 supplement pavement design process.

- Accessible via the web and mobile devices
- Free — no cost to use
- Based on AASHTO pavement design equations
- User-friendly
- Share, save, and print project designs
- Interactive help and resource links

www.PaveXpressDesign.com



29

Perpetual Pavement Design Software

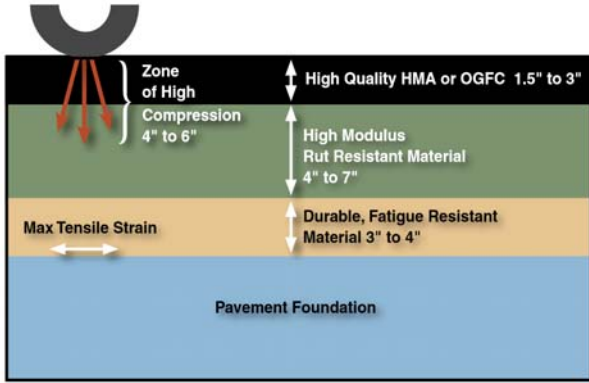


PerRoad is uses the mechanistic-empirical design philosophy. The program couples layered elastic analysis with a statistical analysis procedure (Monte Carlo simulation) to estimate stresses and strains within a pavement. In order to predict the strains which would prove detrimental for fatigue cracking or structural rutting.

www.asphaltroads.org/perpetual-pavement

30

What does a P. P. look like?



31



Ya But, Ya But
Does
Asphalt
Really Last ?

32



HIGHWAY 090 E RURAL HIGHWAY 090 E
Beginning MRM = 98.14 Beginning MRM = 98.14
Ending MRM = 102.00 Ending MRM = 102.00



STRUCTURAL DESCRIPTION	THK	FD	FD
Shouldr Type - Prim/Sec	AC/N/D	AC/N/D	AC/N/D
Surf Width	24 (024)	24 (024)	24 (024)
Lf Shldr Width - Prim/Sec	4 (04) / (0)	4 (04) / (0)	4 (04) / (0)
Rt Shldr Width - Prim/Sec	0 (00) / (0)	10 (10) / (0)	10 (10) / (0)
Widths-RDWY/ROW-Freedom/Min	032/470/470	038/470/470	038/470/253
Roadbed Layer 1	2015/AS3/1.3	2015/AS3/1.3	2015/AS3/1.3
Roadbed Layer 2	2002/TC/0.0	2002/TC/0.0	2002/TC/0.0
Roadbed Layer 3	1999/AH3/1.5	1999/AH3/1.5	1999/AH3/1.5
Roadbed Layer 4	1999/AH3/3.5	1993/AG3/2.0	1999/AH3/1.5
Roadbed Layer 5	1977/AG4/3.5	1993/AG3/3.0	1977/AG4/2.0
Roadbed Layer 6	1964/AB3/4.5	1993/AG3/3.0	1969/AG3/2.0
Roadbed Layer 7	1964/BB3/4.0	1993/BS5/12.0	1969/AG3/2.0
Roadbed Layer 8	1964/BU5/4.0		1969/BU8/12.0
Roadbed Layer 9	1964/BU5/6.0		

33



HIGHWAY 090 W RURAL HIGHWAY 090 W
Beginning MRM = 124.23 Beginning MRM = 124.23
Ending MRM = 142.00 Ending MRM = 142.00



STRUCTURAL DESCRIPTION	WBL	WBL	EBL	EBL
Shouldr Type - Prim/Sec	AC/N/D	AC/N/D	AC/N/D	TKSID
Surf Width	24 (024)	24 (024)	4 (04) / (0)	4 (04) / (0)
Lf Shldr Width - Prim/Sec	10 (10) / (0)	10 (10) / (0)	10 (10) / (0)	8 (08) / (0)
Rt Shldr Width - Prim/Sec	4 (04) / (0)	4 (04) / (0)	038/200/200	038/250/200
Widths-RDWY/ROW-Freedom/Min	038/220/220	038/220/220	1074/113/113	2014/CD1/10.5
Roadbed Layer 1	2012/TM/0.4	2012/TM/0.4		
Roadbed Layer 2	2002/AS3/1.3	2002/AS3/1.3		2014/BS4/5.0
Roadbed Layer 3	2002/AH3/1.5	2002/AH3/1.5		
Roadbed Layer 4	1990/AR/1.2	1974/AG3/3.0		
Roadbed Layer 5	1974/AG3/1.8	1974/AG3/9.0		
Roadbed Layer 6	1974/AG3/1.8	1974/BL3/12.0		
Roadbed Layer 7	1974/BL3/12.0			
Roadbed Layer 8				

34



HIGHWAY 115 RURAL HIGHWAY 115
Beginning MRM = 104.13 Beginning MRM = 104.13
Ending MRM = 107.51 Ending MRM = 107.51



STRUCTURAL DESCRIPTION	THK	FD
Shouldr Type - Prim/Sec	AC/N/D	AC/N/D
Surf Width	24 (024)	
Lf Shldr Width - Prim/Sec	6 (06) / (0)	
Rt Shldr Width - Prim/Sec	6 (06) / (0)	
Widths-RDWY/ROW-Freedom/Min	036/200/200	
Roadbed Layer 1	2008/TS3/0.4	
Roadbed Layer 2	1999/TC/0.0	
Roadbed Layer 3	1990/TS3/0.5	
Roadbed Layer 4	1986/AG3/2.0	
Roadbed Layer 5	1975/TS3/0.5	
Roadbed Layer 6	1967/AG3/3.0	
Roadbed Layer 7	1967/AG3/3.0	
Roadbed Layer 8	1967/BU8/12.0	

35



How it looks today



36



Lift Thickness and Mix Type

Design - Section 3



Mix Type



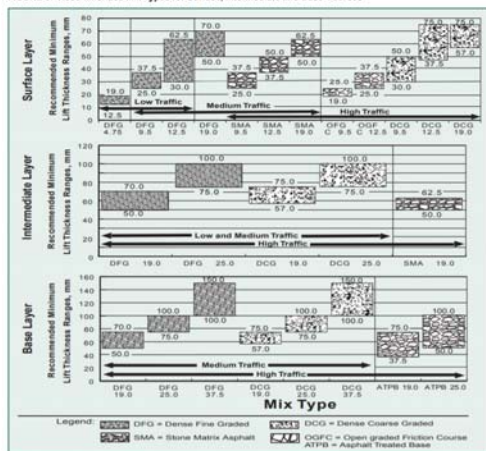
<https://www.fhwa.dot.gov/publications/research/infrastructure/pavements/asphalt/HMA.pdf>

38



Recommended Lift Thickness

FIGURE 3: Recommended Mix Types for Surface, Intermediate and Base Courses



39



Mix Type

NMAS grading is different than older "Topsize" Grading

Old Rule of Thumb - Minimum lift thickness = 2x Topsize

NMAS - Minimum compacted thickness

- ✓ 4 times nominal aggregate size
- ✓ 3 times nominal aggregate size for fine graded mixtures

- Thicker lifts are easier to compact
- Cool slower providing longer compaction time

Minimum -----NOT MAXIMUM !



Specified Mix Gradations

TABLE 460-1 AGGREGATE GRADATION MASTER RANGE AND VMA REQUIREMENTS

SIEVE	PERCENT PASSING DESIGNATED SIEVES						
	NOMINAL SIZE						
	No. 1 (37.5 mm)	No. 2 (25.0 mm)	No.3 (19.0 mm)	No. 4 (12.5 mm)	No. 5 (9.5 mm)	SMA No. 4 (12.5 mm)	SMA No. 5 (9.5 mm)
50.0-mm	100						
37.5-mm	90-100	100					
25.0-mm	90 max	90-100	100				
19.0-mm		90 max	90-100	100		100	
12.5-mm			90 max	90-100	100	90-97	100
9.5-mm				90 max	90-100	58-72	90-100
4.75-mm					90 max	25-35	35-45
2.36-mm	15-41	19-45	23-49	28-58	32-67	15-25	18-28
75-µm	0-6.0	1.0-7.0	2.0-8.0	2.0-10.0	2.0-10.0	8.0-12.0	10.0-14.0
% MINIMUM VMA	11.0	12.0	13.0	14.0 ⁽¹⁾	15.0 ⁽²⁾	16.0	17.0

⁽¹⁾ 14.5 for LT and MT mixes.

⁽²⁾ 15.5 for LT and MT mixes.



Field Tolerances

460.2.8.2.1.5 Control Limits

⁽¹⁾ Conform to the following control limits for the JMF and warning limits based on a running average of the last 4 data points:

ITEM	JMF LIMITS	WARNING LIMITS
Percent passing given sieve:		
37.5-mm	+/- 6.0	+/- 4.5
25.0-mm	+/- 6.0	+/- 4.5
19.0-mm	+/- 5.5	+/- 4.0
12.5-mm	+/- 5.5	+/- 4.0
9.5-mm	+/- 5.5	+/- 4.0
2.36-mm	+/- 5.0	+/- 4.0
75-µm	+/- 2.0	+/- 1.5
Asphaltic content in percent	-0.3	-0.2
Air voids in percent ⁽¹⁾	+1.3/-1.0	+1.0/-0.7
VMA in percent ⁽²⁾	-0.5	-0.2

⁽¹⁾ For SMA, JMF limits are +/-1.3 and warning limits are +/-1.0.

⁽²⁾ VMA limits based on minimum requirement for mix design nominal maximum aggregate size in [Table 460-1](#).

⁽³⁾ Warning bands are defined as the area between the JMF limits and the warning limits.

Lift Thickness



NCAT Test Track 1st Cycle

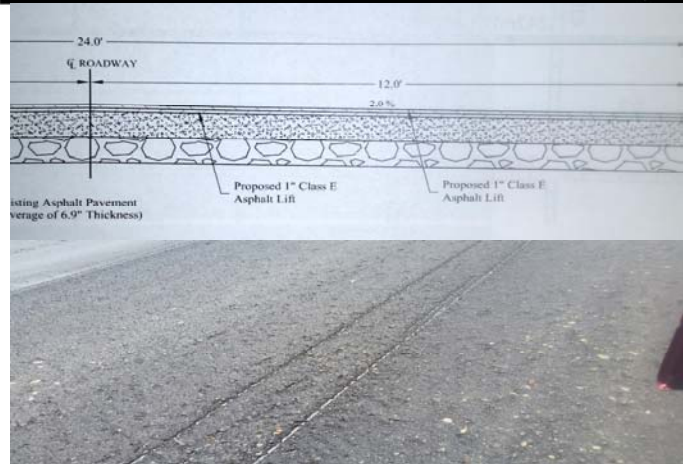


Limestone Fine	Gravel Intermed.	Slag & Lms Fine	Granite Fine	Granite Coarse	Granite Intermed.
Limestone Fine	Gravel Coarse	Slag & Lms Coarse	Gravel Coarse	Granite Intermed.	Granite Fine

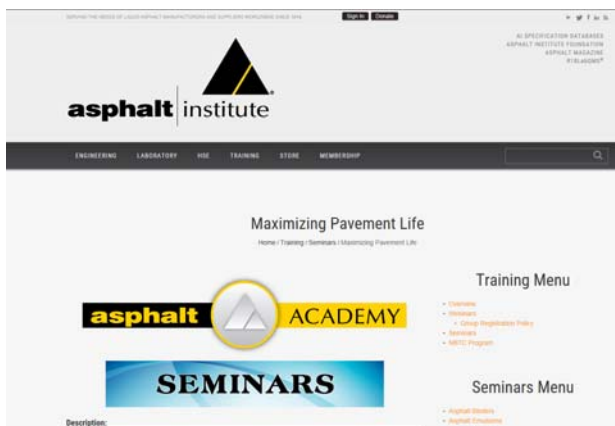
Coarse, intermediate, and fine gradations. **No differences in rutting performance!**

Courtesy of NCAT

Lift Thickness



New 2-day Workshop in 2019



Thank you to our Members



Thank you for your attention



Discussion/Questions?